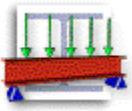
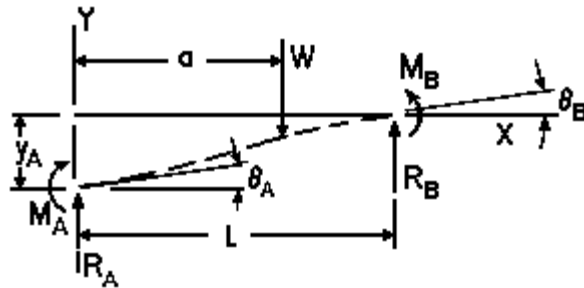


Table 3 Shear, Moment, Slope and Deflection Formulas
for Elastic Straight Beams

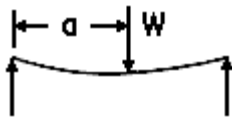


Case 1e Concentrated Intermediate Load; Left End Simply Supported, Right End Simply Supported

Concentrated intermediate load



Left end simply supported, right end simply supported



Notation file

Provides a description of Table 3 and the notation used.

**Enter dimensions,
properties and
loading**

Before progressing further, calculate the moment of inertia (I) for your cross section by flipping to Table 1. Enter the computed value below:

Table 1

Area moment of inertia:	$I \equiv 6.944 \cdot \text{in}^4$
Length of beam:	$L \equiv 10 \cdot \text{ft}$
Height of beam:	$h \equiv 4 \cdot \text{in}$
Distance from left edge to load:	$a \equiv 5 \cdot \text{ft}$
Modulus of elasticity:	$E \equiv 30 \cdot 10^6 \cdot \frac{\text{lbf}}{\text{in}^2}$
Load:	$W \equiv 1500 \cdot \text{lbf}$
Yield Strength	$\sigma_y := 30 \text{ksi}$

Boundary values

The following specify the reaction forces (R), moments (M), slopes (θ) and deflections (y) at the left and right ends of the beam (denoted as A and B, respectively).

At the left end of the beam (simply supported):

$$R_A := \frac{W}{L} \cdot (L - a) \quad R_A = 750 \cdot \text{lbf}$$

$$M_A := 0 \cdot \text{lbf} \cdot \text{in}$$

$$\theta_A := \frac{-W \cdot a}{6 \cdot E \cdot I \cdot L} \cdot (2 \cdot L - a) \cdot (L - a) \quad \theta_A = -0.371 \cdot \text{deg}$$

$$y_A := 0 \cdot \text{in}$$

At the right end of the beam (simply supported):

$$R_B := \frac{W \cdot a}{L} \quad R_B = 750 \cdot \text{lbf}$$

$$M_B := 0 \cdot \text{lbf} \cdot \text{in}$$

$$\theta_B := \frac{W \cdot a}{6 \cdot E \cdot I \cdot L} \cdot (L^2 - a^2) \quad \theta_B = 0.371 \cdot \text{deg}$$

$$y_B := 0 \cdot \text{in}$$

$$x := 0 \cdot L, .01 \cdot L .. L$$

x ranges from 0 to L, the length of the beam.

$$x_1 := \frac{L}{2}$$

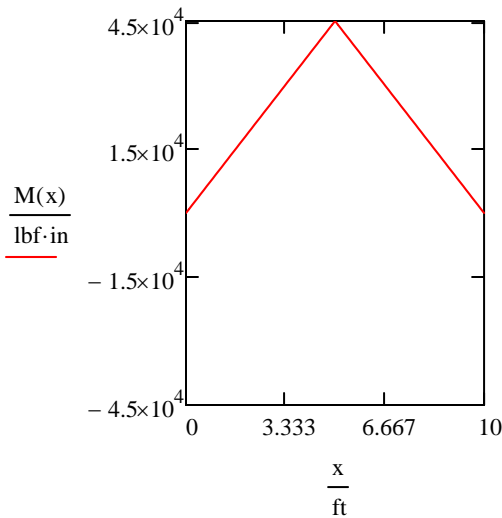
Midpoint of the beam

Bending moment

$$M(x) := M_A + R_A \cdot x - (x > a) \cdot (x - a) \cdot W$$

$$M(x_1) = 4.5 \times 10^4 \cdot \text{lbf} \cdot \text{in}$$

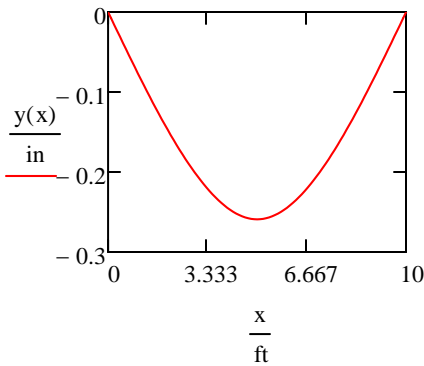
$$M(5\text{ft}) \rightarrow 3750 \cdot \text{ft} \cdot \text{lbf}$$



Deflection

$$y(x) := y_A + \theta_A \cdot x + \frac{M_A \cdot x^2}{2 \cdot E \cdot I} + \frac{R_A \cdot x^3}{6 \cdot E \cdot I} - (x > a) \cdot \left[\frac{W}{6 \cdot E \cdot I} \cdot (x - a)^3 \right]$$

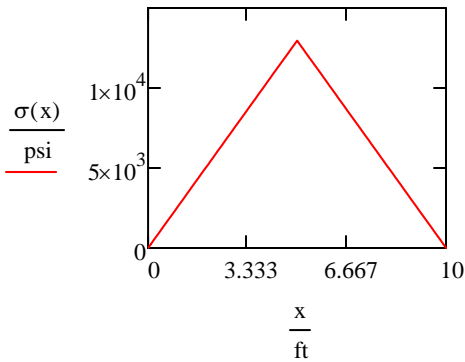
$$y(x_1) = -0.259 \cdot \text{in}$$



Stress

$$\sigma(x) := \frac{M(x) \cdot \frac{h}{2}}{I}$$

$$\sigma(x_1) = 1.296 \times 10^4 \cdot \text{psi}$$



$$F_{\text{max}} := \frac{4 \cdot I \cdot \sigma_y}{L \cdot \frac{h}{2}} = 3.472 \times 10^3 \cdot \text{lbf}$$