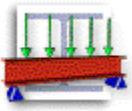
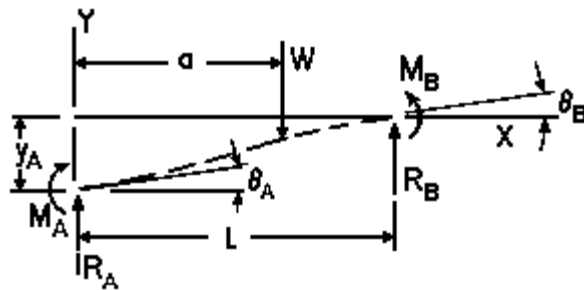


Table 3 Shear, Moment, Slope and Deflection Formulas  
for Elastic Straight Beams

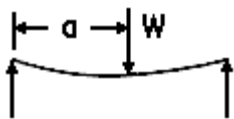


## Case 1e Concentrated Intermediate Load; Left End Simply Supported, Right End Simply Supported

Concentrated intermediate load



Left end simply supported, right end simply supported



### Notation file

Provides a description of Table 3 and the notation used.

**Enter dimensions,  
properties and  
loading**

Before progressing further, calculate the moment of inertia (I) for your cross section by flipping to Table 1. Enter the computed value below:

**Table 1**

Area moment of inertia:	$I \equiv 3.495 \cdot \text{in}^4$
Length of beam:	$L \equiv 10 \cdot \text{ft}$
Height of beam:	$h \equiv 3 \cdot \text{in}$
Distance from left edge to load:	$a \equiv 5 \cdot \text{ft}$
Modulus of elasticity:	$E \equiv 30 \cdot 10^6 \cdot \frac{\text{lbf}}{\text{in}^2}$
Load:	$W \equiv 1500 \cdot \text{lbf}$
Yield Strength	$\sigma_y := 30 \text{ksi}$

## Boundary values

The following specify the reaction forces (R), moments (M), slopes ( $\theta$ ) and deflections (y) at the left and right ends of the beam (denoted as A and B, respectively).

At the left end of the beam (simply supported):

$$R_A := \frac{W}{L} \cdot (L - a) \quad R_A = 750 \cdot \text{lbf}$$

$$M_A := 0 \cdot \text{lbf} \cdot \text{in}$$

$$\theta_A := \frac{-W \cdot a}{6 \cdot E \cdot I \cdot L} \cdot (2 \cdot L - a) \cdot (L - a) \quad \theta_A = -0.738 \cdot \text{deg}$$

$$y_A := 0 \cdot \text{in}$$

At the right end of the beam (simply supported):

$$R_B := \frac{W \cdot a}{L} \quad R_B = 750 \cdot \text{lbf}$$

$$M_B := 0 \cdot \text{lbf} \cdot \text{in}$$

$$\theta_B := \frac{W \cdot a}{6 \cdot E \cdot I \cdot L} \cdot (L^2 - a^2) \quad \theta_B = 0.738 \cdot \text{deg}$$

$$y_B := 0 \cdot \text{in}$$

$$x := 0 \cdot L, .01 \cdot L .. L$$

x ranges from 0 to L, the length of the beam.

$$x_1 := \frac{L}{2}$$

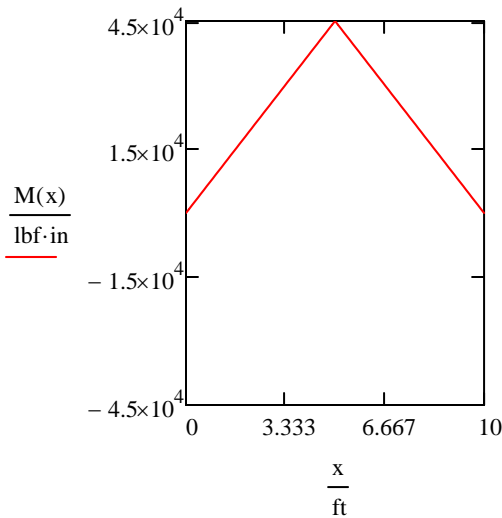
Midpoint of the beam

Bending moment

M(x) := M\_A + R\_A · x - (x > a) · (x - a) · W

M(x\_1) = 4.5 × 10^4 · lbf · in

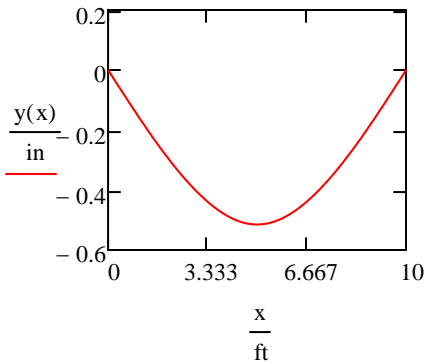
M(5ft) → 3750 · ft · lbf



Deflection

y(x) := y\_A + θ\_A · x + M\_A · x^2 / (2 · E · I) + R\_A · x^3 / (6 · E · I) - (x > a) · [ W / (6 · E · I) · (x - a)^3 ]

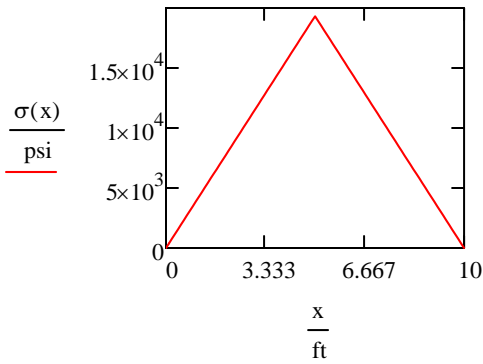
y(x\_1) = -0.515 · in



Stress

σ(x) := M(x) · h / 2I

σ(x\_1) = 1.931 × 10^4 · psi



F\_max := (4 · I · σ\_y) / (L · h / 2) = 2.33 × 10^3 lbf